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TEHNIČKE MERE ZA REHABILITACIJU U HIDROTEHNIČKOM TUNELU NA SASKA RIJECI – M. KAMENICA

Rezime:

Hidrotehnički tuneli predstavljaju izazovne konstrukcije sa stanovišta dizajna, izgradnje i održavanja. Obično su pod konstantnim dejstvom vode, zbog čega su skloni popravljanju i sanaciji. Funkcionalnost ovih vrsta tunela zavisi od odgovarajućih tehničkih mera koje su preduzete u njihovom eksploatacionom periodu. Ovo uključuje znanje i stručnjake iz nekoliko naučnih oblasti. U ovom radu su prikazane neke od tehničkih mera za rehabilitaciju hidrotehničkog tunela na reci Sasa.

Key words: Hidrotehnički tunel; Analiza; Rehabilitacija; PLAXIS 3D

TECHNICAL MEASURES FOR THE REHABILITATION IN THE HYDROTECHNICAL TUNNEL ON SASKA RIVER – M. KAMENICA

Hydrotechnical tunnels represent challenging constructions from a design, construction and maintenance point of view. They are usually under constant effect of the water, which makes them prone to repairs and rehabilitation. The functionality of these kind of tunnels depends on the proper technical measures taken in their service period. This involves knowledge and experts from a few scientific areas. In this paper some of the technical measures for the rehabilitation of the hydrotechnical tunnel on Saska River are shown.

Key words: Hydrotechnical tunnel; Analysis; Rehabilitation; PLAXIS 3D

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1. INTRODUCTION

This tunnel is part of the mine for lead and zinc Sasa, which is one of the biggest and most famous mines in Macedonia, located in the eastern part of the country (Figure 1). It is approximately 12 km from the town Makedonska Kamenica at elevation of 1000 m.

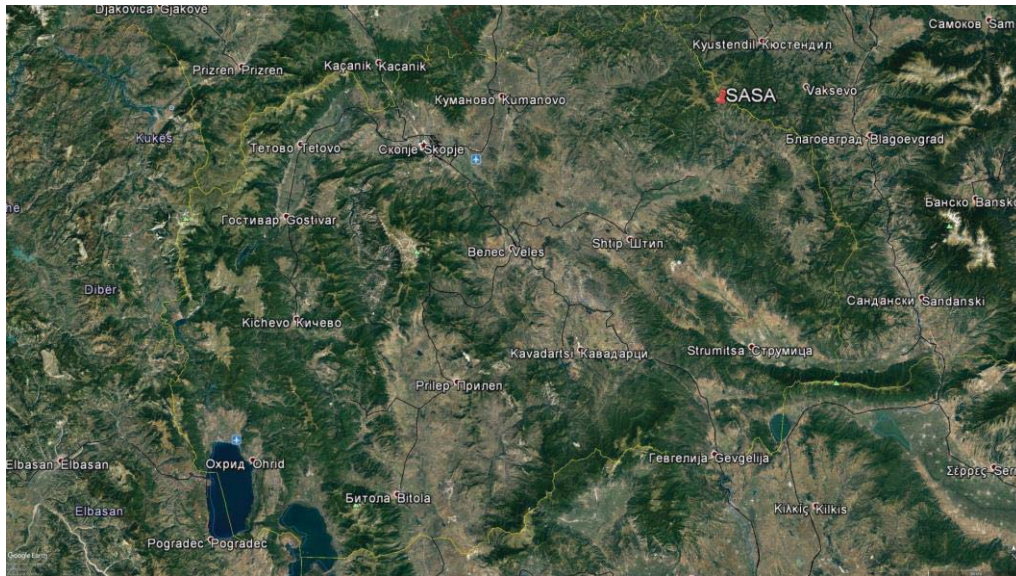


Figure 1 - Satellite view of the mine location (source: Google Earth)

1.1. GENERAL INFORMATION

The tunnel on Saska River is in function since 1971. It has a length of 1925 m with longitudinal slope of 5-7 %. The cross section varies in height and width (≈ 3 m) based on the geological composition and the support lining. Along the tunnel at a certain part are the tailings of the mine. The tailings are used for the disposal of the flotation pulp gained from the technological process of flotation of the minerals of lead and zinc. In the past three tailings have been realized, and another one is planned for the future exploitation period of the mine.

1.2. PREVIOUS REHABILITATIONS

There are several previous interventions made to this tunnel, that include: rehabilitation of the damaged concrete support lining, increasing durability and anti-corrosive protection of the concrete, injection of cracks and holes in the concrete and in the area between the rock and the concrete support, rehabilitation of the dilatation and the working joints, rehabilitation and protection of the invert arch, placement of additional shotcrete and rock bolts etc.

2. GEOLOGICAL AND GEOTECHNICAL PROPERTIES OF THE TERRAIN IN THE ZONE OF THE TUNNEL

The information for the geological condition of the terrain in the zone of the tunnel are collected from a few investigation stages, from which an appropriate base of geotechnical data was created.

2.1. LITHOLOGY

Present in the zone of the tunnel are the following natural and artificial compositions:

- Natural materials: gneiss rock, diluvial, proluvial and scree materials and alluvium (sandy gravel);
- Artificial materials: tailings sand (floatation pulp) and concrete from the tunnel support lining.

2.2. TECTONIC PROPERTIES

The character of the river valley and the condition and interaction of the represented rock masses indicate an intensive tectonic activity of this terrain in the geological past. The results from the investigations from which the faults and the fault zones were verified, also prove the complex tectonic condition of the terrain. The wider region belongs to zones with intensity of $I = VIII^{\circ}$ MCS (Mercali-Cancani-Sieberg). According to the national seismogenic sources the closest located area (berovsko-pehcevska) is with expected intensity of $I = XI^{\circ}$ MCS, and expected magnitude of $M = 7,5^{\circ}$.

2.3. ENGINEERING – GEOLOGICAL TYPES OF ROCK MASSES

There are two groups in which the represented rock masses in the zone of the tunnel can be divided:

- Strongly tied rock masses – represented by the gneisses which build the base of the terrain;
- Loose rock masses – represented by the alluvial sediments and the floatation pulp.

2.4. HYDROGEOLOGICAL PROPERTIES

The represented rock masses according to their hydrogeological function in the terrain assembly behave as collectors and isolators.

In the group of hydrogeological collectors belong the alluvial sediments and the floatation pulp (tailings).

In the group of hydrogeological isolators belong the strongly tied rock masses (gneisses).

2.5. GEOTECHNICAL CHARACTERISTICS OF THE ROCK MASSES

Most of the information's for the characteristics of the rock masses were obtained from the existing technical documentation.

- Gneisses: $RQD=30\%$, $\sigma_p=30$ MPa, $c=300$ KPa, $\phi=36^{\circ}$, $\gamma=25$ kN/m³, $\nu=0.30$, RMR= III class;

- Gravel: $E=52000 \text{ kN/m}^2$, $\nu=0.28$, $c=0 \text{ KPa}$, $\phi=34^\circ$, $\gamma=22.18 - 22.80 \text{ kN/m}^3$;
- Flotation pulp: $c=0 \text{ KPa}$, $\phi=31^\circ$, $\gamma=18 - 20.5 \text{ kN/m}^3$.



Figure 2 – Tunnel exit

3. CURRENT STATE OF THE TUNNEL

From the completely made analysis, the on-site inspection in the tunnel, and the overall results it has been noted that the hydrotechnical tunnel on Saska River can adequately perform its function.

However, due to the observed irregularities along the tunnel in terms of established and identified defects, it was recommended to apply measures with which the functionality of the tunnel construction would increase, and this would result in better condition and longer exploitation period for using the tunnel.

Mostly the irregularities refer to increased amount of water on the walls of the tunnel and some damages to the support lining. A specific defect which is present along the tunnel is the damage of the concrete invert. Along the length of the tunnel almost the entire bottom where the water flows is eroded and in some places a channel with a depth of over 20 cm was formed.

4. REHABILITATION SOLUTIONS AND ANALYSIS OF THE TUNNEL

The tunnel is divided in few sections:

- Parts with primary (rock bolts and reinforced shotcrete) and secondary support (reinforced concrete lining);
- Parts with only primary support (rock bolts and reinforced shotcrete);
- Parts with no support (only rock mass).

The rehabilitations solutions vary depending on the sections.

4.1. REHABILITATION OF THE TUNNEL SECTIONS WITH ONLY PRIMARY SUPPORT

Parts of the tunnel have only primary support (reinforce shotcrete $d=20$ cm and rock bolts $L_a=2,5$ m $a_p/a_n=1,25/1,25$ m) and invert arch with no secondary support. For these segments multiple variant solutions where considered:

- Solution 1: Partial secondary support – pillars (reinforced concrete $d=30$ cm);
- Solution 2: Full secondary support above the invert (reinforced concrete $d=30$ cm);
- Solution 3: Rehabilitation of the invert with anchors, new reinforcement and new layer of concrete ($d=20$ cm).

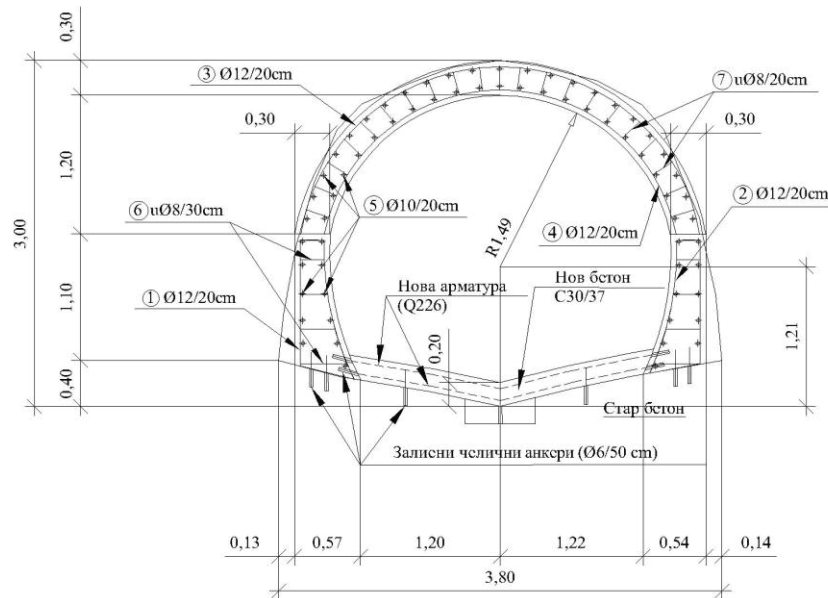


Figure 3 – Tunnel cross section with full secondary support

4.2. ANALYSIS OF THE TUNNEL SECTIONS WITH ONLY PRIMARY SUPPORT

The analysis was made for multiple variant solutions covering several stages:

- Solution 1: Analysis of the tunnel with the invert from reinforced concrete and primary support from reinforced shotcrete and rock bolts (for stability control);
- Solution 2: Analysis of the tunnel with the invert and pillars from reinforced concrete, as well as primary support from reinforced shotcrete and rock bolts;
- Solution 3: Analysis of the tunnel with primary and full secondary support (d=30 cm);
- Solution 4: Analysis of the tunnel with primary and secondary support, without the invert.

The analysis process was simulated in the numerical model in multiple phases:

- Phase 1: Initial stress condition in the massif before construction;
- Phase 2: Excavation of the tunnel and construction of the invert with the pillars;
- Phase 3: Pseudo-static analysis.

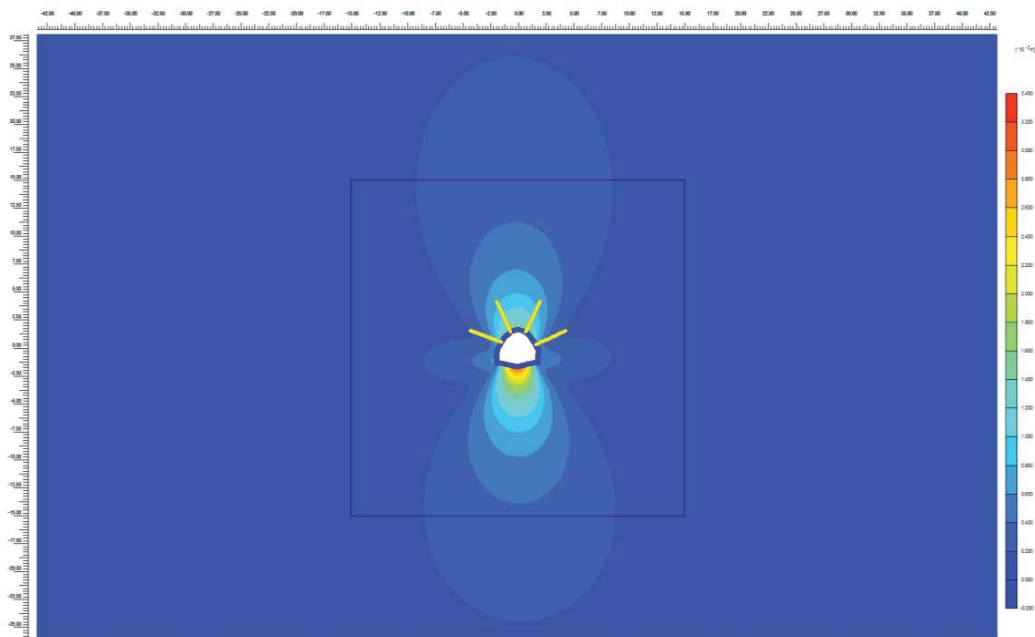


Figure 4 – Deformations of the local environment and the lining

The maximum deformations in the local environment are 3.24 mm. The results from the analysis show that the bigger static values occur in seismic conditions.

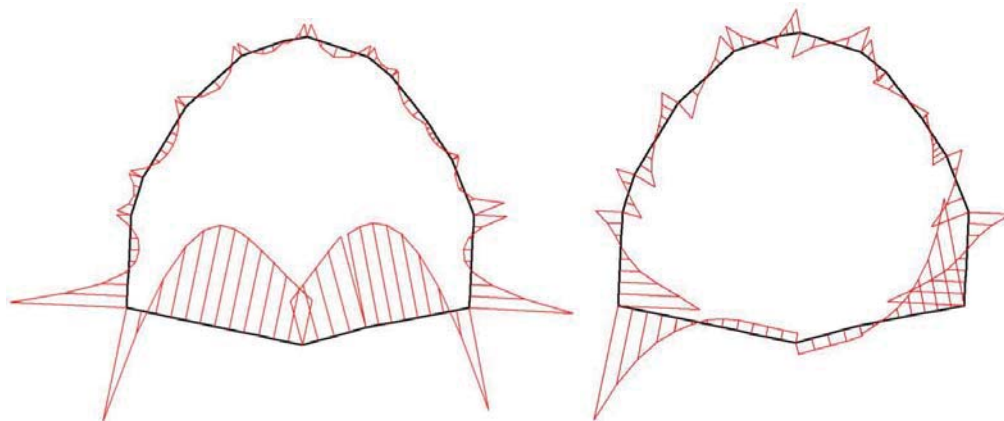


Figure 5 – Diagrams of bending moment (left) and transversal forces (right)

5. CONCLUSIONS

For proper exploitation and long lasting service life, the rehabilitation and maintenance is mandatory process for hydrotechnical tunnels. The rehabilitation solutions and analysis for the tunnel on Saska River shown in this paper are just part of the complexed process that enables the improvement of the tunnel construction bearing capacity and functionality. This process requires qualified experts from different scientific areas and adequate detailed information's regarding the local environment, previous conditions, rehabilitations and interventions and the current state of the tunnel.

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